

IMPORTANT NOTES

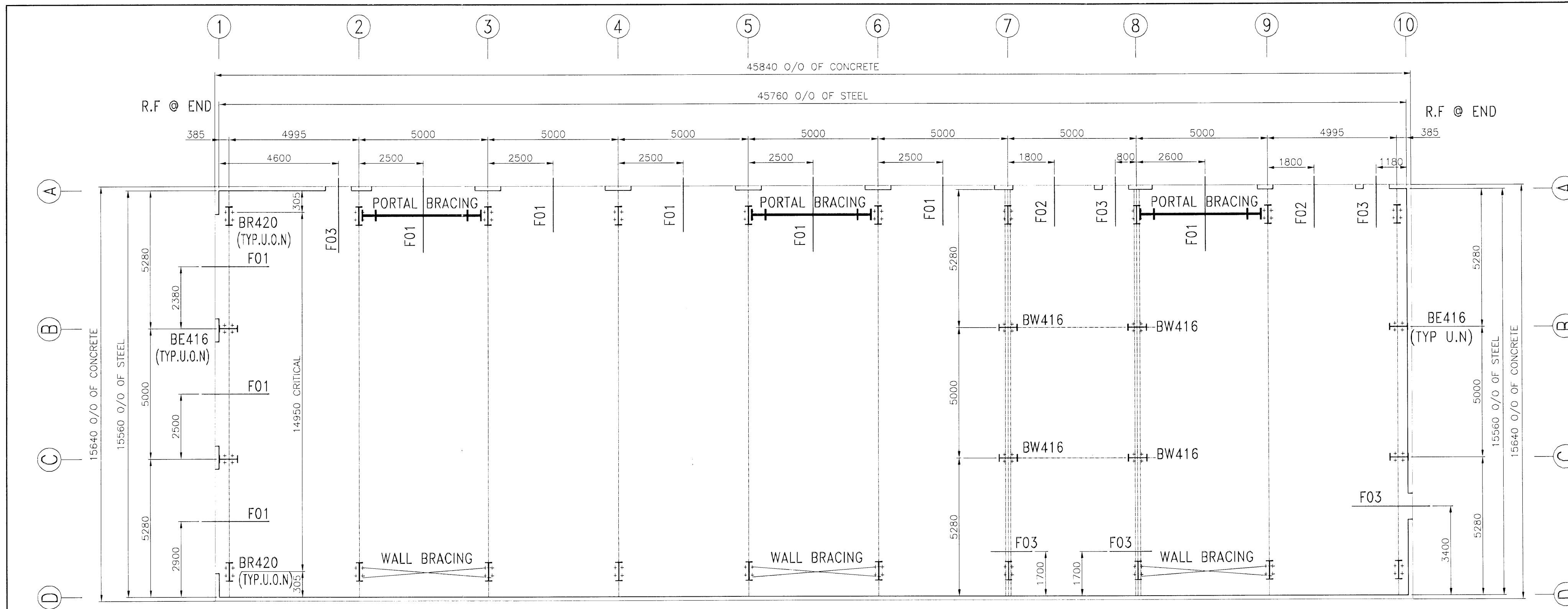
- Bracing reactions are to be considered with the main frame reactions. All reactions and loads are in Kilo-Newton and in Meter units.
- Zamil Steel assumes no responsibility or liability for the column foundations, slab design or construction. Foundations should be designed as a minimum for the given reaction loads.
- The slab or edge beam should be square and level to within 5mm tolerance. Anchor bolts should be set to the given dimensions and projection, with maximum deviation of 5mm. Anchor bolt threads should be protected during concreting operation, or thoroughly cleaned after pouring. All templates should be removed.
- The customer is responsible for accurate locations of building lines and bench marks at the site of the building.
- The installation of anchor bolts and embedded items must be done in accordance with the Code of Standard Practice for Steel Buildings of AISC section 7 and Zamil Steel drawings subject to the following maximum allowable tolerances:
 - 3mm center to center of any two bolts within an anchor bolt group.
 - 6mm center to center of adjacent anchor bolt groups.
 - Elevation of the top of anchor bolts +/- 12mm.
 - Maximum accumulation of 6mm per 30 meters along the established column line of multiple anchor bolt groups but not to exceed a total of 25mm.
 - 6mm from the center of any anchor bolt group to the established column line through that group.
- The tolerances of paragraphs b, c and d apply to offset dimensions shown on the plans, measured parallel and perpendicular to the nearest established column line for individual columns shown on the plans to be offset from established column line.
- Anchor bolts shall be set perpendicular to the theoretical bearing surface unless shown otherwise.
- Anchor bolts should be set by the owner (or his civil contractor) in accordance with the "Issued for Construction" Anchor Bolt Setting Plan. Zamil Steel shall not be responsible for consequences arising from casting the anchor bolts on the basis of the "Approval drawings".

Samþykkt þann

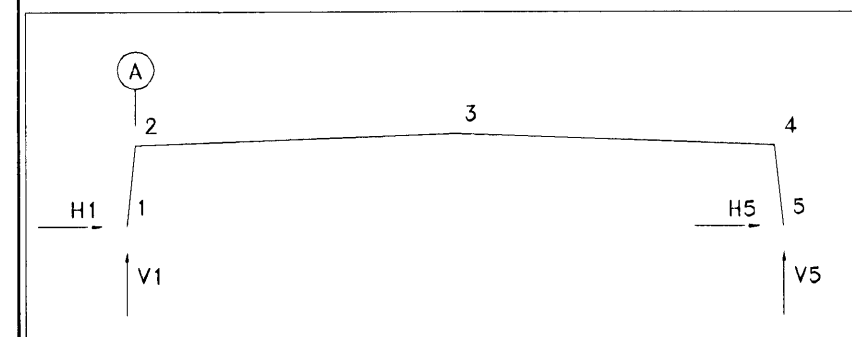
01 DEC 2008

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ANCHOR BOLT KEY PLAN



STRUCTURE REACTIONS
ASFADFRM/WZ-D568*1-3.MAC

Load Cases	V1 (kN)	H1 (kN)	V5 (kN)	H5 (kN)	Frames at Grid Lines
DEAD	16	5	15	-5	1-10
DEAD_SR	4	5	-4	5	1-10
DEAD_SL	-4	-5	4	-5	1-10
LIVE	23	10	23	-10	1-10
SNOWD	24	11	24	-11	1-10
SNOW11	18	6	6	-6	1-10
SNOW12	4	1	1	-1	1-10
SNOW21	6	6	18	-6	1-10
SNOW22	1	1	4	-1	1-10
TEMP1	0	1	0	-1	1-10
TEMP2	0	-1	0	1	1-10
WINDEND	-81	-14	-81	14	1-10
WINDLEFT	-90	-58	-44	-9	1-10
WINDRIGHT	-44	9	-90	57	1-10
SNOWD_SR	2	2	-2	2	1-10
SNOW11_SR	1	1	-1	1	1-10
SNOW12_SR	1	1	-1	1	1-10
SNOW21_SR	1	1	-1	1	1-10
SNOW22_SR	1	1	-1	1	1-10
SNOWD_SL	-2	-2	2	-2	1-10
SNOW11_SL	-1	-1	1	-1	1-10
SNOW12_SL	-1	-1	1	-1	1-10
SNOW21_SL	-1	-1	1	-1	1-10
SNOW22_SL	-1	-1	1	-1	1-10

LOAD CASES	REACTIONS (KN)	
	H1	V1
DEAD	-1	4
LIVE	0	0
WIP	-35	0
WIS	35	0

End Wall Column Reactions at Grid Line No.1

LOAD CASES	REACTIONS (KN)	
	H1	V1
DEAD	-1	4
LIVE	0	0
WIP	-35	0
WIS	35	0

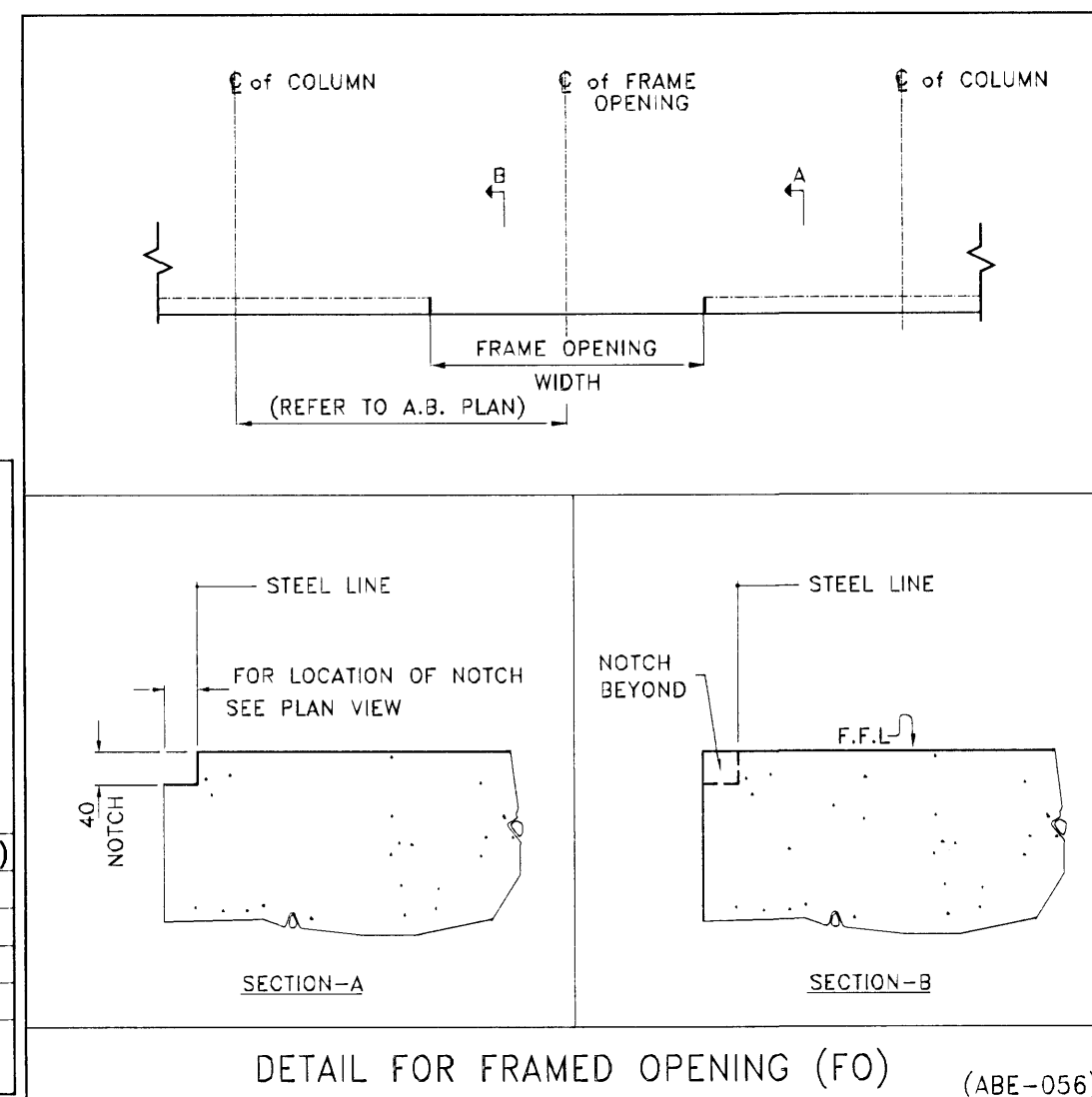
End Wall Column Reactions at Grid Line No.10

LOAD CASES	REACTIONS (KN)	
	H1	V1
DEAD	0	2
WIND	10	25

Bracing Reactions at Grid Line No.A

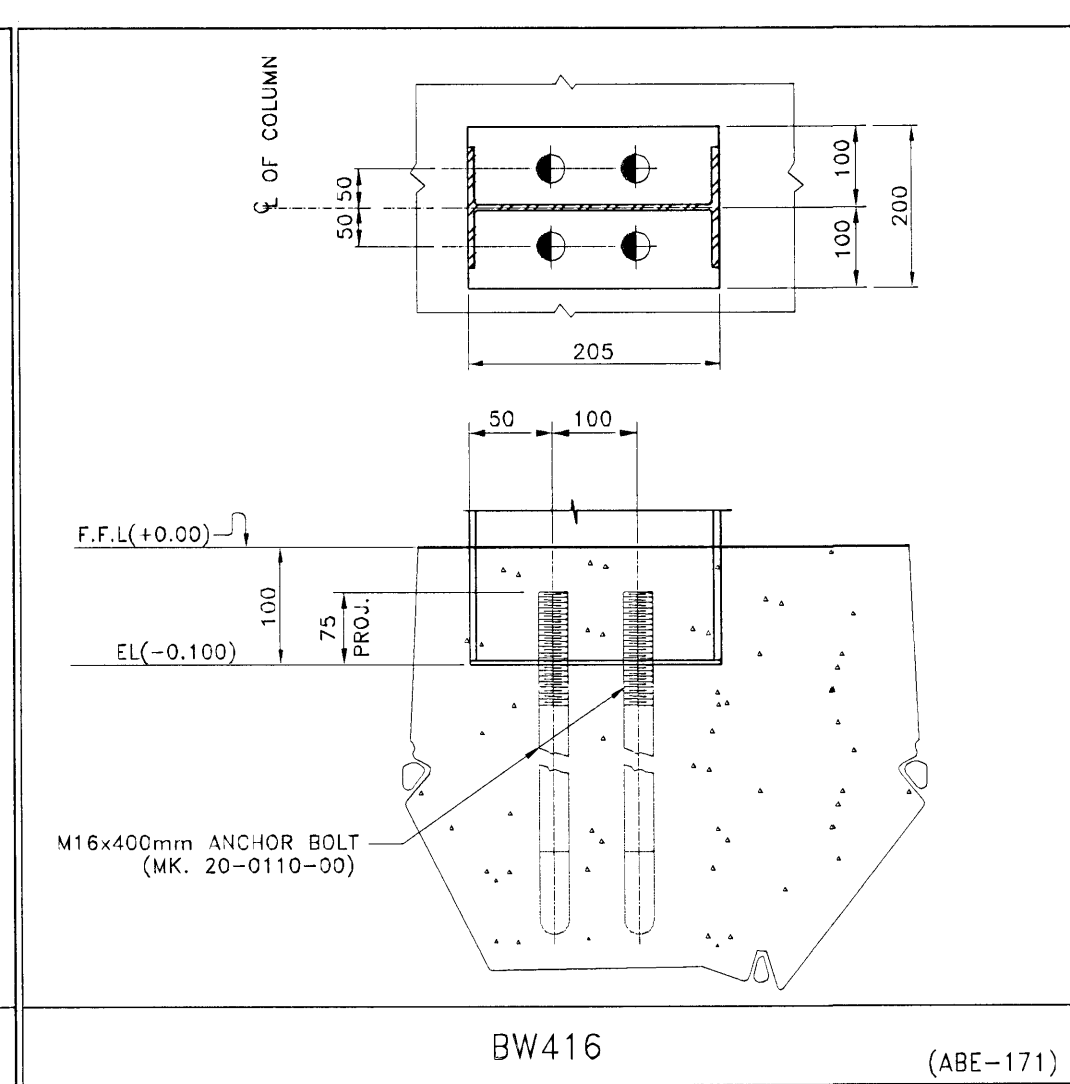
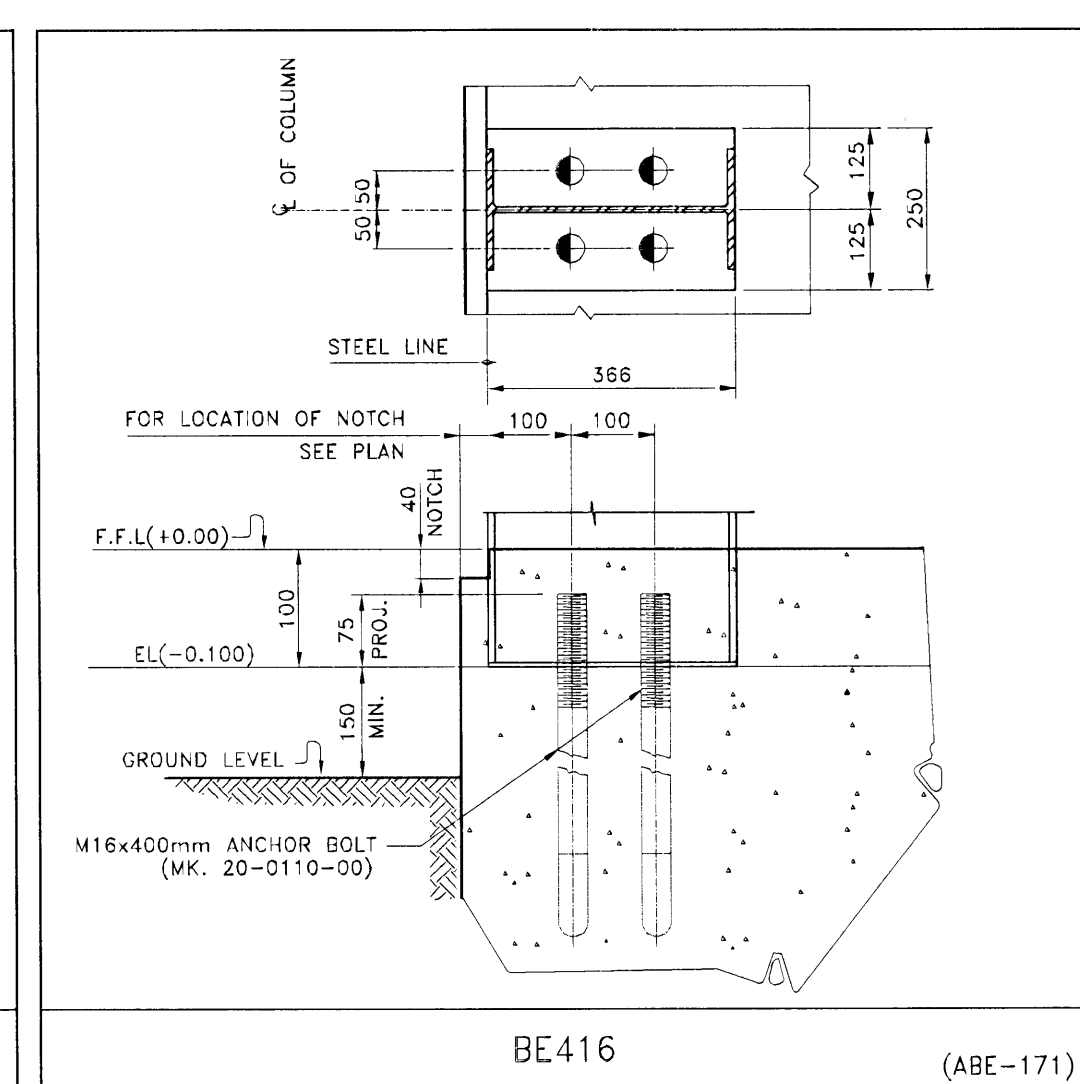
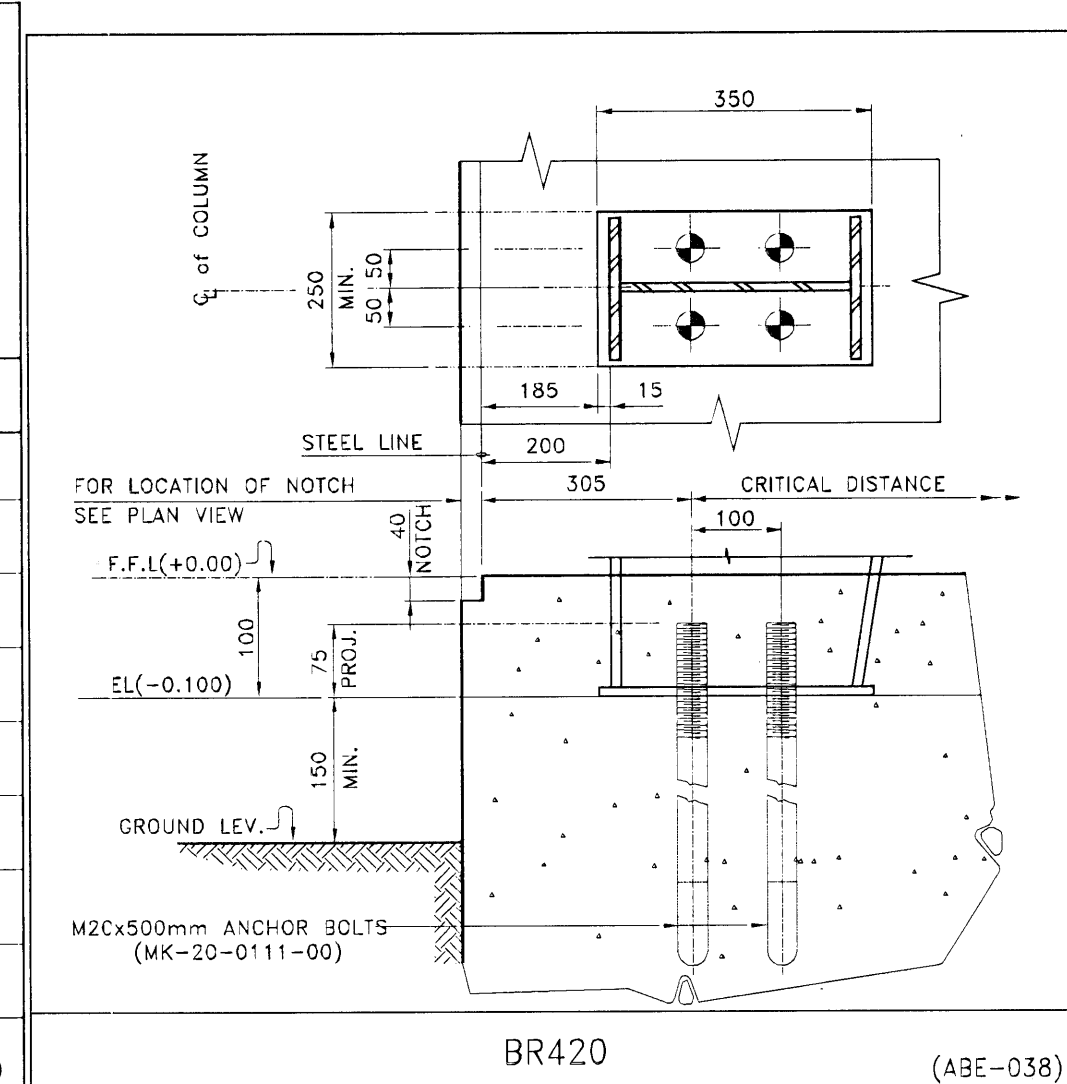
LOAD CASES	REACTIONS (KN)	
	H1	V1
WIND	20	24

Bracing Reactions at Grid Line No.D



QUAN.	SYM.	SIZE	PART MARK	A (mm)	B (mm)	C (mm)	R (mm)	D (mm)
32	●	M16	20-0110-00	400	90	80	24	100
80	●	M20	20-0111-00	500	110	100	30	125
	●	M24	20-0112-00	600	140	128	40	125
	⊗	M30	20-0113-00	900	170	150	50	175
	⊗	M36	20-0114-00	1000	210	192	60	175

ANCHOR BOLT SCHEDULE (ABE-057)



ACCESS TYPE	WIDTH	HEIGHT
F01	4000	4000
F02	3200	4000
F03	1000	2200

3	04.05.2008	ISSUED FOR CONSTRUCTION	ASU	KSK	TMM
2	18.03.2008	RE-ISSUED FOR APPROVAL	HKS	REN	TMM
1	27.11.2007	RE-ISSUED FOR APPROVAL	HKS	REN	RMR
0	08.10.07	ISSUED FOR APPROVAL	SAK	REM	RMR
NO.	DATE	DESCRIPTION	DRN	CHK	DSN

ENGINEERING DEPARTMENT

ZS JOB NO. : V7-D568 BUILDING NO.: 01
PROJECT NAME : SIBERIA HOUSE
CUSTOMER : HEDINN
JOBSITE LOCATION : HAFNAFIRDI, ICELAND
DRAWING TITLE : ANCHOR BOLT PLAN DRAWING NO. : FO1